

## Lecture 2

(2)

- How do granular materials respond to stress? Why are stresses localized?

1. How do we think about this question for elastic materials?

$$\nabla \cdot \sigma = \overset{\rightarrow}{f_{ext}}$$

$$\begin{aligned} \text{2D} \quad \partial_y \sigma_{yy} + \partial_x \sigma_{xy} &= f_y \\ \partial_y \sigma_{yx} + \partial_x \sigma_{xx} &= f_{x\text{ext}} \end{aligned}$$

AND  $\sigma_{xy} = \sigma_{yx}$  (torque balance).

2 eqns for 3 unknowns.

$$\sigma_{ij} = C_{ijkl} \epsilon_{kl} \quad \text{strain tensor}$$

$$\epsilon_{ij} = \frac{1}{2} \left[ \frac{\partial u_i}{\partial x_j} + \frac{\partial u_j}{\partial x_i} \right]$$

Where  $\vec{u}$  is the displacement field measured from a reference state (zero stress)

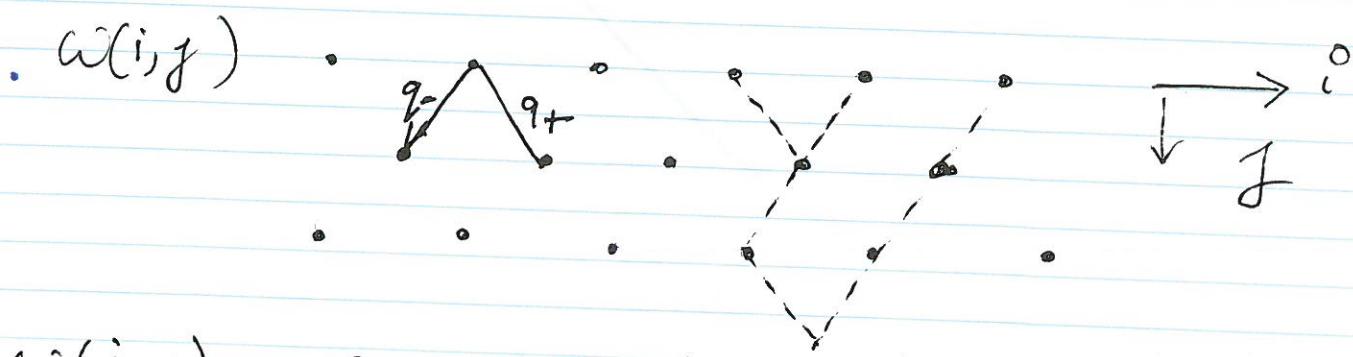
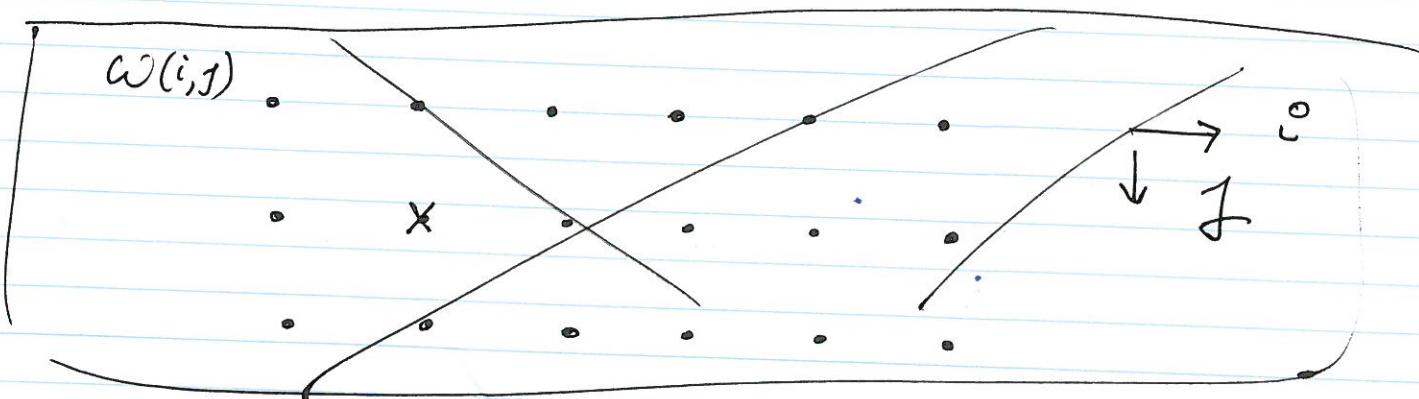
Compatibility Condition

$$\begin{aligned} \partial_y^2 u_{xx} + \partial_x^2 u_{yy} - 2 \partial_x \partial_y u_{xy} &= 0 \\ \left[ \partial_y^4 + t \partial_x^4 + 2 \partial_x \partial_y^2 \right] \sigma_{ij} &= 0 \end{aligned}$$

The lack of a reference state  $\Rightarrow$  these closure relations are not available to us.

### Stress - only approach

#### $q$ -model



$$\begin{aligned}\omega(i,j) = \omega_o &+ q_+(i-1, j-1) \omega(i-1, j-1) \\ &+ q_-(i+1, j-1) \omega(i+1, j-1)\end{aligned}$$

Ordered  $q_+ = q_- = \frac{1}{2}$

$$\omega(i,j) = \omega_o + \frac{1}{2} [\omega(i-1, j-1) + \omega(i+1, j-1)]$$

Master eqn for random walks in space " $i$ " and time " $j$ "

(4)

$$\frac{dP(n,t)}{dt} = -\sum m_{n,m} P(n,t) + \sum m_{m,n} P(m,t)$$

$q_+$  or  $q_- = 1$  the other is zero.

coalescing random walks

- Imposes scalar force balance
- Disorder is coded in the  $q$ 's [contact loading length]
- Uniform distribution of  $q$ 's  
(sort of an Edwards assumption)
- Advantage  $\Rightarrow$  solve for  $P(\omega)$

for a uniform dist'n of  $q$ 's

$$] \rightarrow \infty \quad P(\omega) = \frac{\omega}{\langle \omega^2 \rangle} e^{-\frac{\omega}{\langle \omega \rangle}}$$

Exponential tail [from uniform dist'n]

- Exponential tail is robust as long as we don't take the  $q=0,1$  case.

[Bouchaud lectures : Liu et al  
Science, PRE ~ 2002]

(5)

The scalar model  $\rightarrow$  continuum limit.

$$\partial_t w = f + \partial_{xx}^2 w \quad \text{if } q_{\pm} = \frac{1}{2}$$

Diffusion.

$$\text{add disorder } q_{\pm} = (\pm \omega(i,j)) / 2$$

small  $\epsilon$

$$\Rightarrow D_R = D_0 + \text{corr}^2 \text{ corr}$$

that depends on size  
of  $\epsilon$ .

Response of a point force applied  
at the top of a pile

- like diffusion of a  $\delta$ -fn

$\rightarrow$  Broadens

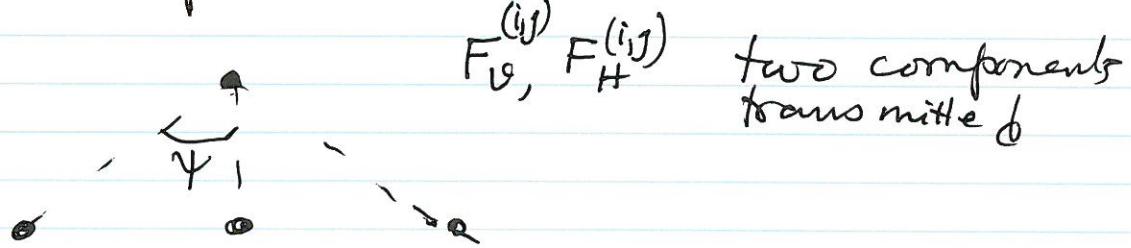


no "chains"

- Missing vectorial force - balance.

(continuing in this vein of thinking)

some simple models



- assume force gets transmitted only over stirn of leg (frictionless)

(6)

2. a fraction  $\rho$  of  $F_H$  gets transferred along middle leg

$F_V$  :  $\rho$  along middle

$(1-\rho) \left( \frac{1+\epsilon}{2} \right)$  along right.

$(1-\rho) \left( \frac{1-\epsilon}{2} \right)$  along left.

Balance of horizontal force on  $(i,j)$

$$\text{requires } \epsilon = \frac{1}{(1-\rho) \tan \psi} \frac{F_H(i,j)}{F_V(i,j)}$$

$F_H$  was absent in 9 mosc.

- $\epsilon \neq 0 \Rightarrow q_+$  and  $q_-$  have to be different
- bias propagation of  $F_V$  along  $F_H$

$$\partial_y F_V + \partial_x F_H = f + \alpha \frac{(1-\rho)}{2} \partial_{yy}^2 F_V$$

$$\partial_y F_H + \partial_x [C_0^2 F_V] = \frac{\alpha}{2} \partial_{xx}^2 F_H$$

$$C_0^2 = (1-\rho) \tan^2 \psi$$

- eliminating  $F_H \rightarrow \left[ \partial_y^2 - C_0^2 \partial_x^2 \right] F_V = 0$
- Wave equation!

(7)

- characteristics

$$x - c_0 y, \quad x + c_0 y$$

along which the wave propagates

$\Rightarrow$  force chains as soon as you ~~not~~ include vector force by.

- ~~Any stress component reln of the form~~

- Connect to

$$\partial_y \sigma_{yy} + \partial_x \sigma_{xy} = f$$

$$\partial_y \sigma_{yx} + \partial_x \sigma_{xx} = 0$$

if ignore  $\partial_{xx}^2, \partial_{yy}^2$  in  $F_H, F_V$  eqn.

- $\sigma_{xy} = F_H, \quad \boxed{\sigma_{xx} = c_0^2 \sigma_{yy}}$

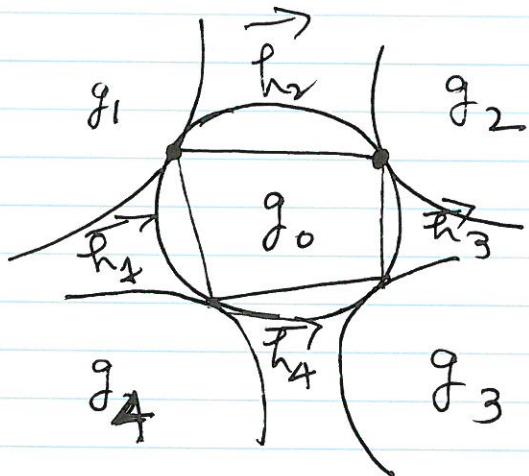
- Any closure of 1stn form

$$\sigma_{xx} = c_0^2 \sigma_{yy} g\left(\frac{\sigma_{xy}^2}{\sigma_{yy}^2}\right)$$

leads to wave-like propagation.

- Breaks rotational symmetry.

We have the tools to do better!



$$\vec{f}_{g_0, g_1}, \vec{f}_{g_0, g_2}, \vec{f}_{g_0, g_3}, \vec{f}_{g_0, g_4}$$

$$\vec{f}_{g_0, g_1} = \vec{h}_1 - \vec{h}_2 + \vec{\phi}_{g_1} - \vec{\phi}_{g_0}$$

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$$\vec{f}_{g_0, g_4} = \vec{h}_4 - \vec{h}_1 + \vec{\phi}_{g_4} - \vec{\phi}_{g_0}$$

$$= -\vec{f}_{g_0}^{\text{body}} = (\quad) + ?$$

Fails if we have body forces such as gravity

$$\sum \vec{f}_{g_0, g_i} = -\vec{f}_{g_0}^{\text{body}} \neq 0$$

- Each contact has two loops and two grains involved. Can we introduce auxiliary fields on the grains?

(9)

$$-\vec{f}_{g_0}^{\text{body}} = \vec{\phi}_{g_1} + \vec{\phi}_{g_2} + \vec{\phi}_{g_3} + \vec{\phi}_{g_4} - 4\vec{\phi}_{g_0}$$

This is the discrete Laplacian

$$-\vec{f}_{g_0}^{\text{body}} = \square^2 \vec{\phi}_{g_0}$$

Reminder 9 model  $\partial_t \omega = \partial_x^2 \omega$

Define a ket  $|\phi\rangle = \{\vec{\phi}_1 \dots \vec{\phi}_N\}$

and  $|\vec{f}^{\text{body}}\rangle = \{\vec{f}_1^{\text{body}} \dots \vec{f}_N^{\text{body}}\}$

$$\boxed{\square^2 |\phi\rangle = -|\vec{f}^{\text{body}}\rangle}$$

$$\square^2 = \begin{bmatrix} -z_1 & 0 & 1 & \dots & 1 & \dots \\ 0 & -z_2 & \ddots & \ddots & \ddots & \ddots \end{bmatrix} \quad z_1 \text{ entries are } 1 \\ z_2 \text{ entries are } 1$$

Graph Laplacian [Graph with loops]

Real-space network captured by  $\square^2$

- Response to  $\alpha$   
depends on
- How localized  
then is  $\alpha$   
nature of  $\square^2$
- $\square^2 = \sum_{i=1}^N \delta$
- One zero e  
with  $|\lambda_1\rangle =$
- All other  $\lambda$
- Eliminate  
 $|\tilde{f}\rangle = |f\rangle$
- Can invert  $\square^2$   
 $(\square^2)^{-1} \square^2 =$

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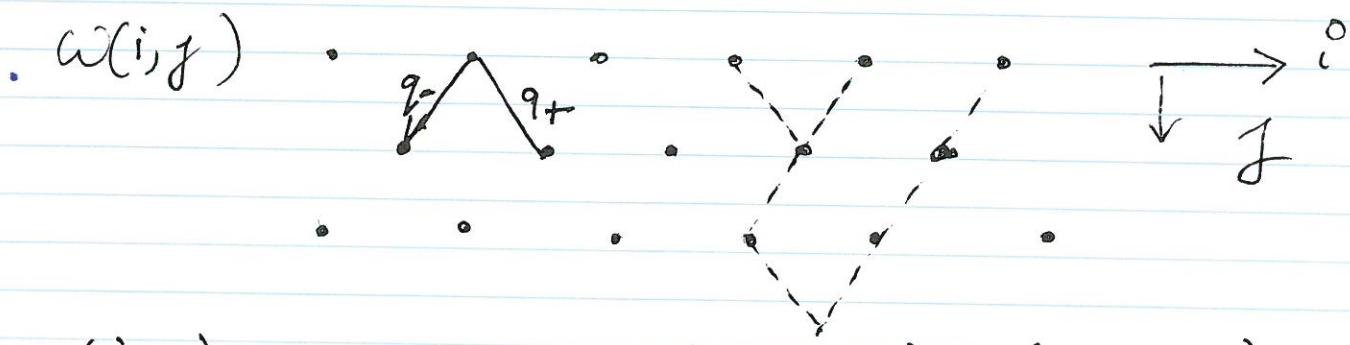
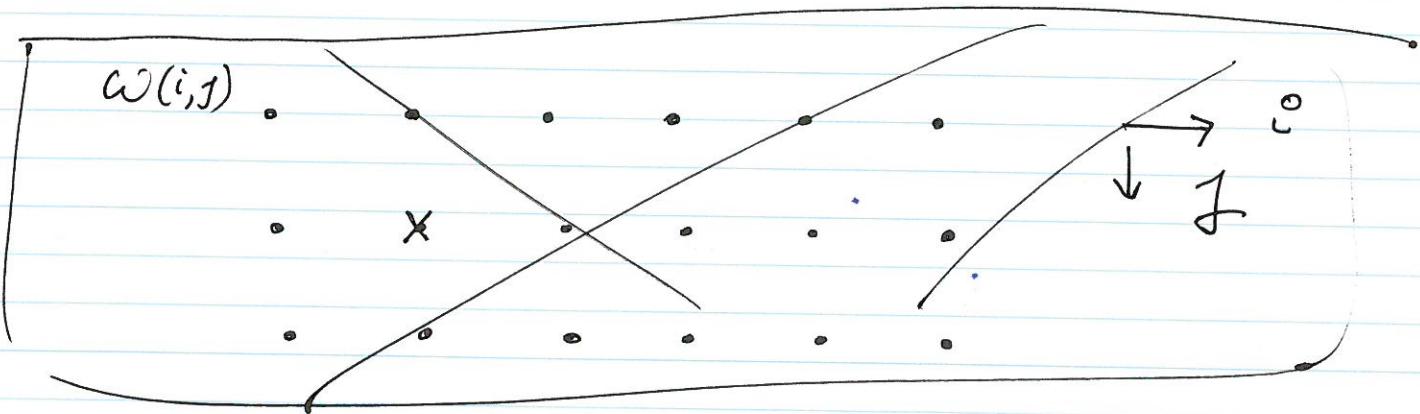
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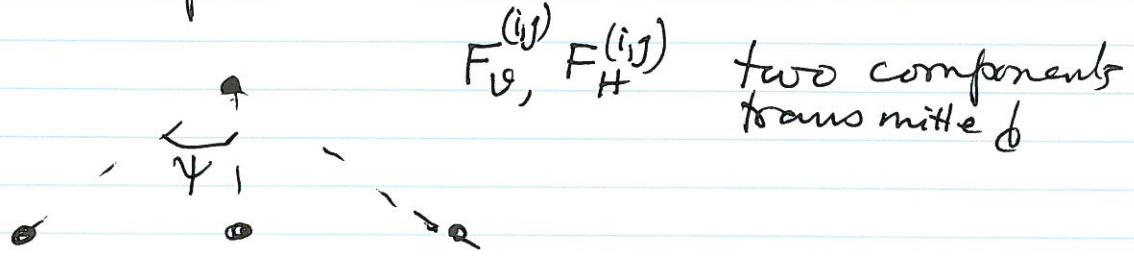
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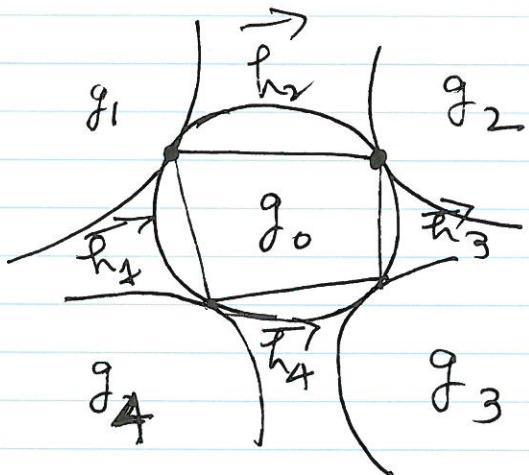
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Graph Laplacian [Graph with loops]  
 Real-space network captured by  $\square^2$

(10)

- Response to a given  $|f^{\text{body}}\rangle$   
depends on the spectrum of  $\square^2$
- How localized the response is then is determined by the nature of the eigenfunc of  $\square^2$

$$\square^2$$

- $\square^2 = \sum_{i=1}^N \lambda_i | \lambda_i \rangle \langle \lambda_i |$

- One zero eigenvalue.

with  $|\lambda_1\rangle = \{1, \dots, 1\}$

because  
of row sum  
 $= 0$

- All other  $\lambda$  are -ve.

- Eliminate  $|\lambda_1\rangle$  by

$$|\tilde{f}\rangle = |f^{\text{body}}\rangle - \frac{1}{N} \sum_{i=1}^N |\lambda_i| |f^{\text{body}}\rangle$$

- Can invert <sup>after</sup> ~~by~~ project out zero m.

- $|\phi\rangle = -[\square^2]^{-1} |\tilde{f}^{\text{body}}\rangle$

$$(\square^2)^{-1} \square^2 = 1 - |\lambda_1\rangle \langle \lambda_1|$$

$$|\phi\rangle = \sum_{i \neq 1} \frac{1}{x_i} \langle \lambda_i | \tilde{f}^{\text{body}} | \lambda_i \rangle$$

Can construct "new"  $\overset{\rightarrow}{f}_{g_0, g_1}$

from  $|\phi\rangle$

NOTE : Whenever I mentioned the "Coulomb Condition" it referred to the constraint  $f_t \leq \mu f_N$  : nothing to do with E&M ! Sorry for not explaining this granular terminology. Mohr-Coulomb failure in granular solids is the original model of failure where the condition above is violated.